# Chapter 3 <br> Elementary Fluid Dynamics Bernoulli Equation 

PART 1
"Fluid Mechanics has helped me build a stronger problemsolving skill by helping me not focus on the problem itself, but on the path that must be followed."

Former student, Winter 2022


## Educational Objectives

- Explain the development, uses, and limitations of the Bernoulli Equations.
- Use the Bernoulli equation (stand-alone or in combination with the simple continuity) to solve flow problems.
- Apply the concepts of static, stagnation, dynamic, and total pressures.


## Elementary Fluid Dynamics

- To understand fluid dynamics, fundamental laws that govern motion of fluid particles must be considered
- Newton's second law ( $\mathrm{F}=\mathrm{ma}$ ) can be applied to motion of fluid particles
- Bernoulli's equation is one of the oldest fluid mechanics equation that can be effectively used to predict and analyze flow
- Bernoulli's equation is also "the most used and most abused equation in fluid mechanics"


Bernoulli (1667-1748)

## Elementary Fluid Dynamics

- When a fluid particle moves from one point to another, it experiences an acceleration or deceleration
- The net force on the fluid particle is: $\mathrm{F}=\mathrm{ma}$
- In this chapter we assume fluid with zero viscosity (INVISCID FLUIDS)


Newton
(1642-1727)

- In reality there is no inviscid fluid since every fluid supports shear stress
- In many flow situations, the viscous effect is smaller than other effects and can be neglected


## Fluid Dynamics- Visual representation of a flow

Streamline: Streamlines are lines drawn in the flow field so that at a given instant they are tangent to the direction of flow at every point in the flow field. Consequently,
$\left|\frac{d y}{d x}\right|_{\text {streamline }}=\frac{v}{u}$

Since the streamlines are tangent to the velocity vector at every point in the flow field, there can be no flow across a streamline. Streamlines are the most commonly used visualization technique in fluid mechanics.

## Visual representation of fluid flow

Boundary Layer Separation and Vortex Shedding

| Streamline: Commonly used method <br> visualization | of flow |
| :--- | :--- | :--- |



## Flow Induced Vibration



Play (k)

## Elementary Fluid Dynamics

- The motion of each fluid particle is described in terms of its velocity vector $V$

(a)
- If nothing changes with time at a $\vec{a}=a_{x} \hat{i}+a_{y} \hat{j}=a_{s} \hat{s}+a_{n} \hat{n}$ given location in flow field, it is called STEADY FLOW
- For steady flow, each particles slides along its path, the velocity vector is tangent to the path

- The lines that are tangent to the velocity vectors are called $a_{s}=\frac{\partial}{\partial t}=V \frac{\partial}{\partial s} ; a_{n}=\frac{V^{2}}{R}$ STREAMLINES


## Bernoulli's Equation

$$
\sum \delta F_{s}=(\delta m) a_{s}
$$

$$
\begin{aligned}
& \sum \delta F_{s}=\delta F_{\text {gravity }}+\delta F_{\text {pressure }}+\delta F_{\text {viscous }}(=0) \\
& \Rightarrow \sum \delta F_{s}=(-\gamma(\delta \forall) \sin \theta)+\left(-\frac{\partial p}{\partial s}(\delta \forall)\right)+0 \\
& \delta m=\rho(\delta \forall) \\
& a_{s}=\frac{\partial V}{\partial t}=V \frac{\partial V}{\partial s}
\end{aligned}
$$

## Bernoulli's Equation

Put it all together

$$
\sum \delta F_{s}=(\delta m) a_{s}
$$



$$
\begin{aligned}
& \Rightarrow(-\gamma \delta \forall \sin \theta)+\left(-\frac{\partial p}{\partial s} \delta \forall\right)=\rho(\delta \forall) \cdot V \frac{\partial V}{\partial s} \\
& \Rightarrow-\gamma \sin \theta+\left(-\frac{\partial p}{\partial s}\right)=\rho V, \frac{\partial V}{\partial s}
\end{aligned}
$$

$$
\sin \theta=\frac{\partial z}{\partial s} \quad \rho V \frac{\partial V}{\partial s}=\frac{\partial}{\partial s}\left(\frac{1}{2} \rho V^{2}\right)
$$

$$
\Rightarrow-\gamma \frac{\partial z}{\partial s}+\left(-\frac{\partial p}{\partial s}\right)=\frac{\partial}{\partial s}\left(\frac{1}{2} \rho V^{2}\right)
$$

$$
p+\gamma z+\frac{1}{2} \rho V^{2}=C
$$



## Class 05: Bernoulli's Equation

## Bernoulli Equation

$$
p+\gamma z+\frac{1}{2} \rho V^{2}=C
$$

Only applicable if:


- Density constant along streamline (Incompressible)
- Steady Flow
- Viscous force negligible compare to the force due to gravity and Pressure (Invisid)
- Only good along a Streamline
- No pump or work


## ISIS + holds $\longrightarrow$ Apply Bernoulli

## Bernoulli's Equation - Interpretation

$\square$ In terms of pressure

$$
p+\gamma z+\frac{1}{2} \rho V^{2}=C, \text { along a streamline }
$$

p Static Pressure

## $\gamma z$

$\frac{1}{2} \rho V^{2}$

Hydrostatic Pressure - Related to potential energy variation due to elevation changes

Dynamic Pressure

## Bernoulli's Equation - Interpretation

$\square$ In terms of heights - called heads

$$
\frac{p}{\gamma}+z+\frac{V^{2}}{2 g}=C, \text { along a streamline }
$$

Pressure head - represents the height of a column of the fluid that is needed to produce the pressure $p$.
$Z$ Elevation head - elevation term, related to the potential energy of the fluid particle.
$\overline{2 g}$ for the fluid to fall freely (neglecting friction) if it is to $2 g$ reach velocity $V$ from rest.

Bottomline: Sum of the heads is constant along a streamline

## Hydraulic \& Energy Grade lines

## $\square$ In terms of Energy

$$
\frac{p}{\gamma}+z+\frac{V^{2}}{2 g}=C, \text { along a streamline }
$$



Piezometric Head
$H G L=\frac{P}{\gamma}+Z \quad$ Potential Energy?
$E G L=H G L+\frac{V^{2}}{2 g}$
Kinetic Energy?


## DEFINITION

## BERNOULLI represents the CONSERVATION of ENERGY or "TOTAL PRESSURE " along a STREAMLINE between "ANY" two points.

It can be applied for the following conditions:
Incompressible
Steady Flow
INVISCID (Ignore Friction)
Streamline
NO SHAFT WORK Between Points

## Bernoulli's Equations - Example

Problem \# 1: Air flows through a nozzle and exits to the atmosphere as shown in the Figure below. If the pressure at the upstream location is read as 100 kPa and the velocity at the upstream is known to be about $30 \mathrm{mph}(13 \mathrm{~m} / \mathrm{s})$, what is the velocity at the nozzle exit?

Solution: Consider 2 points along a streamline - one point at the upstream and another one is at the exit.
Given: $\mathrm{P}_{1}=100 \mathrm{kPa}, \mathrm{V}_{1}=13 \mathrm{~m} / \mathrm{s}, \mathrm{P}_{2}=\mathbf{0}$
Apply Bernoulli at $\mathbf{P}_{1}$ and $\mathbf{P}_{\mathbf{2}}$ :

$$
\begin{aligned}
& P_{1}+\frac{1}{2} \rho V_{1}^{2}+\gamma Z_{1}=P_{2}+\frac{1}{2} \rho V_{2}^{2}+\gamma Z_{2} ; Z_{1}=Z_{2}-\text { same elevation; } \rho=1.23 \mathrm{~kg} / \mathrm{m}^{3} \\
& \Rightarrow V_{2}=\sqrt{V_{1}^{2}+2\left(P_{1}-P_{2}\right) / \rho}=443.45 \mathrm{~m} / \mathrm{s}
\end{aligned}
$$

## Stagnation Point

Stagnation point: The point where the fluid particle velocity become stationary, i.e. ZERO VELOCITY. There is a stagnation point on any stationary body placed into a flowing fluid. Some of the fluid flows 'over' and some 'under' the object.

The streamline of the fluid particle has to divide at the stagnation point. The divided line is called stagnation streamline.

Stagnation Pressure:


## DYNAMIC <br> $\overbrace{P_{\text {stag }}}^{\text {TOTAL }}=\overbrace{P_{0}}^{\text {STATIC }}+\overbrace{\frac{1}{2} \rho V_{0}^{2}}$ <br> $V_{0}=\sqrt{\frac{2\left(P_{\text {stag }}-P_{0}\right)}{\rho}}$ <br> 

## Visual representation of fluid flow

## Stagnation point and dividing streamlines



## Stagnation Point

## Example: Stagnation point

- To determine the difference in pressure between point 1 and 2, Bernoulli's equation can be applied along the streamline that passes through point 1 and point 2
- Point (1) to be free stream so that $\mathrm{V}_{1}=\mathrm{V}_{0}$
- Assuming point (1) and (2) are at same elevation $\mathrm{z}_{1}=\mathrm{z}_{2}$
- By measuring the differential pressure $\mathbf{p}_{2}-\mathbf{p}_{1}$, the speed $\mathbf{V}_{0}$ can be determined


$$
\begin{aligned}
& p_{1}+\frac{1}{2} \rho V_{1}^{2}+\gamma z_{1}=p_{2}+\frac{1}{2} \rho V_{2}^{2}+\gamma z_{2} \\
& p_{2}-p_{1}=\frac{1}{2} \rho V_{1}^{2}=\frac{1}{2} \rho V_{0}^{2}
\end{aligned}
$$

If the bicyclist was accelerating or decelerating, the flow would be unsteady $\left(V_{2} \neq 0\right)$ and the above analysis would be incorrect.

## Stagnation Point


(a)

(b)


Stagnation point \& Stagnation Pressure

- There is a stagnation point on any stationary body that is placed into a flowing fluid


## Pitot/Static Tube:




- FIGURE 3.7 Typical Pitot-static tube designs.


## PITOT/STATIC TUBE

## Simple Pitot tube

A pitot tube is used in wind tunnel experiments and on airplanes to measure flow speed. It's a slender tube that has two holes on it. The front hole is placed in the airstream to measure what's called the stagnation pressure. The side hole measures the static pressure.


## Application of the Bernoull's Equation - Pitot Tube

## Pitot Tube:

Apply Bernoulli's equation between two points - point $1 \&$ point 2

$$
\frac{p_{1}}{\gamma}+\frac{V_{1}^{2}}{2 g}+\mathrm{z}_{1}=\frac{p_{2}}{\gamma}+\frac{V_{2}^{2}}{2 g}+\mathrm{z}_{2}
$$

At pt. 2, $V_{2}=0$ (stagnation point)

$$
V_{1}=V=\sqrt{\frac{2}{\rho}\left(p_{z, 1}-p_{z, 2}\right)}
$$

If a differential pressure gage is connected across the taps,

$$
V=\sqrt{2 \Delta p / \rho}
$$

$\Delta p$ pressure difference measured by the gage


## Free Jet

$$
\frac{P_{1}}{\gamma}+\frac{V_{1}^{2}}{2 g}+z_{1}=\frac{P_{2}}{\gamma}+\frac{V_{2}^{2}}{2 g}+z_{2}
$$

FREE SURFACE LARGE TANK

## Free Jet

- The fluid leaves as free jet $\left(p_{2}=0\right)$
- The exit pressure for an incompressible fluid jet is equal to the surrounding pressure ( $p_{2}=0$ )
- The speed increases between point (1) and (5) according to the last equation on the right
- Where $H$ is the distance - the fluid has fallen outside the nozzle
$\mathrm{P}=0, \mathrm{~V}=0$


$$
\begin{aligned}
& \gamma h=\frac{1}{2} \rho V^{2} \\
& V=\sqrt{2 \frac{\gamma h}{\rho}}=\sqrt{2 g h}
\end{aligned}
$$

$$
V=\sqrt{2 g(h+H)}
$$

## Bernoulli's Equations - Example

Problem \# 2: Air flows through a nozzle and static pressure tap placed just before converging section of the nozzle as shown in the Figure below. Determine the velocity of the fluid upstream of the entrance?

Solution: Apply Bernoulli along a streamline through $P_{1}$ and $P_{2}$.
$P_{1}+\frac{1}{2} \rho V_{1}^{2}+\gamma Z_{1}=P_{2}+\frac{1}{2} \rho V_{2}^{2}+\gamma Z_{2}$
( $Z_{1}=Z_{2}$ - same elevation; $\rho=1.23 \mathrm{~kg} / \mathrm{m}^{3}$ )
$\Rightarrow V_{1}=\sqrt{2\left(P_{2}-P_{1}\right) / \rho} ; V_{2}=0 @ P_{2}$ because of stagnation point.
To determine $\mathbf{P}_{\mathbf{2}}-\mathbf{P}_{\mathbf{1}}$, use manometry:

$$
\begin{aligned}
& P_{1}+\gamma_{H g}(0.08 \mathrm{~m})=P_{2} \\
& \Rightarrow P_{2}-P_{1}=133 \times 10^{3}\left(\mathrm{~N} / \mathrm{m}^{2}\right)(0.08 \mathrm{~m})=10,640 \mathrm{~Pa}
\end{aligned}
$$



STAGNATION POINT
$\longrightarrow \Rightarrow V_{l}=\sqrt{(2 \times 10,640 \mathrm{~Pa}) /\left(1.23 \mathrm{~kg} / \mathrm{m}^{3}\right)}=131.5 \mathrm{~m} / \mathrm{s}$

## Bernoulli's Equations - Example

Problem \# 3.48: Air is drawn into a wind tunnel used for testing automobiles as shown in the Figure below. (a) Determine the manometer reading, $h$, when the velocity in the test section is $\mathbf{6 0 m p h}$. Note that there is a 1 -inch column of oil on the water in the manometer. (b) Determine the difference between the stagnation pressure on the front of the automobile and the pressure in the test section.

Solution: Apply Bernoulli along a streamline through $\mathbf{P}_{1}$ and $P_{2}$.
$P_{1}+\frac{1}{2} \rho V_{1}^{2}+\gamma Z_{1}=P_{2}+\frac{1}{2} \rho V_{2}^{2}+\gamma Z_{2}$
( $Z_{1}=Z_{2}$ - same elevation; $P_{1}=0 ; V_{1}=0 ; V_{2}=60 \mathrm{mph}=88 \mathrm{ft} / \mathrm{s}$ )
$\Rightarrow P_{2}=-\frac{1}{2} \rho V_{2}^{2}=-\frac{1}{2}\left(0.00238\right.$ slugs $\left./ f t^{3}\right)(88 \mathrm{ft} / \mathrm{s})^{2}=-9.22 \mathrm{lbf} / \mathrm{ft}^{2}$

## Bernoulli's Equations - Example

## USING manometry we have (IGNORE AIR ABOVE FLUID):

$P_{2}+\gamma_{H_{2} \mathrm{O}} h-\gamma_{\text {oil }}(1 / 12 f t)=P_{0}=0 ; \quad \gamma_{\text {oil }}=S . G . \times \gamma_{H_{2} \mathrm{O}}=0.9 \times\left(62.4 \mathrm{lb} / \mathrm{ft}^{3}\right)=56.2 \mathrm{lb} / \mathrm{ft}^{3}$
$\Rightarrow-9.22 \mathrm{lbf} / f t^{2}+\left(62.4 l b / f t^{3}\right)(h)-\left(56.2 l b / f t^{3}\right)(1 / 12 f t)=0$
$\Rightarrow h=0.223 \mathrm{ft}$
(b) Apply Bernoulli along a streamline through $\mathbf{P}_{2}$ and $\mathbf{P}_{3}$.
$P_{2}+\frac{1}{2} \rho V_{2}^{2}+\gamma Z_{2}=P_{3}+\frac{1}{2} \rho V_{3}^{2}+\gamma Z_{3}$
$\left(Z_{2}=Z_{3}-\right.$ same elevation; $\left.V_{3}=0 ; V_{2}=60 \mathrm{mph}=88 \mathrm{ft} / \mathrm{s}\right)$
$\Rightarrow P_{3}-P_{2}=\frac{1}{2} \rho V_{2}^{2}=\frac{1}{2}\left(0.00238 s l u g s / f t^{3}\right)(88 \mathrm{ft} / \mathrm{s})^{2}=9.22 \mathrm{lbf} / \mathrm{ft}^{2}$


## HOMEWORK

> 3-44,48,57,59,64,65, $71,72,74,75,90,96,97,99$

# Chapter 3 <br> Elementary Fluid Dynamics Bernoulli Equation 

PART 2

## Confined Flows - Mass Conservation

Conservation of Mass: If the fluid flow is steady so that there is no additional accumulation of fluid within the volume, the rate at which the fluid flows into the volume must equal the rate at which it flows out of the volume.

Mass Flow rate


Volumetric
Flow rate
$\Rightarrow \sum \rho *(A * V)_{\text {in }}=\sum \rho *(A * V)_{\text {out }}$
$\Rightarrow \sum Q_{\text {in }}=\sum Q_{\text {out }}$

gallons/s, m ${ }^{3 / s}$

## MASS CONSERVATION

## FIXED MASS SYSTEM:

Time rate of change of system mass $=0$
$\frac{D M_{s y s}}{D t}=0$
where the system mass, $\mathrm{M}_{s y s}=\rho \forall$ is generally expressed as:
$\mathrm{M}_{s y s}=\int_{s y s} \rho\left[\frac{k g}{m^{3}}\right] d \forall\left[m^{3}\right] ;$ or $:$
$\frac{D}{D t}\left[\int_{\text {sys }} \rho\left[\frac{k g}{m^{3}}\right] d \forall\left[m^{3}\right]\right]=0$
and the integration is over the volume of the system.

## MASS CONTINUITY

CONTINUITY - -NON-DEFORMING CONTROL VOLUME :
MASS STORAGE INSIDE
MASS NET TRANSPORT AT CONTROL SURFACE

$$
\begin{aligned}
& \frac{\partial}{\partial t} \int_{c v}(\rho d \forall)+ \\
& \overbrace{\sum \dot{m}_{\text {out }}-\sum \dot{m}_{\text {in }}}=0
\end{aligned}
$$

$\rho d \forall \rightarrow$ MASS
$\frac{\partial}{\partial t} \int_{c v}(\rho d \forall) \rightarrow$ Mass Storage Over Time
STEADY STATE $\rightarrow$ NO MASS STORAGE
$\frac{\partial}{\partial t}\left[\int_{c v}(\rho d \forall)\right]=0 \rightarrow \rho \equiv$ CONSTANT $\rightarrow \frac{d}{d t}(\rho \forall)=0$
$\sum \dot{m}_{\text {out }}-\sum \dot{m}_{\text {in }}=0$
$\dot{m}=\rho A V_{n}=\rho Q:$ MASS FLOW RATE (mass/time)
$V_{n}=$ NORMAL VELOCITY @ SURFACE

## UNITS <br> Definitions + Concepts <br> Manometry \& Hydrostatics (WHY ?) Pressure Conservation: Bernoulli (WHY ?) Mass Conservation/Continuity (WHY ?)

Fluid Mechanics is NOT the big stuff, but rather it is doing the little things right, over and over, and over. We learn small tools each week that are combined with DEINITIONS and concepts to solve more involved problems, week-by-week.
"...one can not build a house until one learns 'how' and 'when' to use a hammer.."
Dr. K. J. Berry
ASME FELLOW


## Determine "ha"



Figure P3. 44
O John Wiley \& Sons, Inc. All rights reserved.
IDENTIFY CRITICAL POINTS
Free Surfaces
Inlet/Exit


Free Jets
Static / Stagnation
Change in Diameters
MISSING DIMENSIONS
$\frac{V_{2}^{2}}{2 g}-\frac{V_{3}^{2}}{2 g}=-h_{b}=-2 m$

## MASS CONSERVATION

$\mathrm{A}_{2} \mathrm{~V}_{2}=\mathrm{A}_{3} \mathrm{~V}_{3}$
$V_{3}=\frac{A_{2} V_{2}}{A_{3}}$
COMBINE
$\frac{V_{2}^{2}}{2 g}\left(1-\left(\frac{\mathrm{A}_{2}}{\mathrm{~A}_{3}}\right)^{2}\right)=-h_{b}$
SOLVE
$V_{2}=\sqrt{\left\{\begin{array}{l}\{\underbrace{}_{<0} \frac{-h_{b}[m] \cdot 2 g\left[\frac{m}{s^{2}}\right]}{1-\left(\frac{\mathrm{A}_{2}}{\mathrm{~A}_{3}}\right)^{2}}\end{array}\right\}\left[\mathrm{m}^{2} / \mathrm{s}^{2}\right]}=1.47 \mathrm{~m} / \mathrm{s}$



0.03-m diameter
0.05-m diameter


$$
\mathrm{Sg}=1.07
$$

## FIND Q



FIND Q

## IDENTIFY CRITICAL POINTS

Free Surfaces
Inlet/Exit
Free Jets
Static / Stagnation
Change in Diameters
MISSING DIMENSIONS

## MANOMETRY--YES

1-4
$\mathrm{P}_{1}+\gamma_{f}(\underline{0.05+10 / 1000}) m+\gamma_{m}(20 / 1000)-\gamma_{f}(20 / 1000+\underline{0.05+10 / 1000})=P_{4}$

$$
\begin{aligned}
& \mathrm{P}_{1}+\frac{20}{1000}(\gamma_{m}-\stackrel{\overbrace{\gamma_{f}}^{\text {water }})=P_{4}}{\mathrm{P}_{1}-P_{4}} \\
& \gamma_{f}
\end{aligned}=\frac{20}{1000}\left(1-\frac{\gamma_{m}}{\gamma_{f}}\right)=0.02\left(1-S_{g_{f}}\right), ~ l
$$



MANOMETRTY:1-4
$\frac{\mathrm{P}_{1}-P_{4}}{\gamma_{f}}=0.02\left(1-S_{g_{f}}\right)$
BERNOULLI: 1-4

$$
\begin{aligned}
& \frac{P_{1}}{\gamma_{f}}+\not f_{1}+\frac{V_{1}^{2}}{2 g}=\frac{P_{4}}{\gamma_{f}}+\not f_{4}+\frac{V_{4}^{2}}{2 g} \\
& \frac{P_{1}}{\gamma_{f}}-\frac{P_{4}}{\gamma_{f}}=\overbrace{\frac{V_{4}^{2}}{2 g}}^{2 g: \frac{V_{1}^{2}}{2 g}}
\end{aligned}
$$




## Flow rate Measurement

- Flowrate through a pipe can be measured by placing some types of restrictions

- Orifice meters
- Nozzle meter
- Venturi meter
- For a given geometry ( $\mathrm{A}_{1}$ and $\mathrm{A}_{2}$ ), the flowrate can be determined if the pressure difference $p_{1}-p_{2}$ is measured


## BERNOULLI

$p_{1}+\frac{1}{2} \rho V_{1}^{2}=p_{2}+\frac{1}{2} \rho V_{2}^{2}$
$p_{1}-p_{2}=\frac{1}{2} \rho V_{2}^{2}-\frac{1}{2} \rho V_{1}^{2}$
$\frac{2\left(p_{1}-p_{2}\right)}{\rho}=V_{2}^{2}-V_{1}^{2}$
MASS CONSERVATION
$\sum \dot{m}_{I N}=\sum \dot{m}_{\text {OUT }} \rightarrow$ CRITICAL PATH FOR AREA CHANGE $\$ \$ \$ \$$
$\not A_{1} V_{1}=\not A_{2} V_{2} \rightarrow Q \equiv$ VOLUME FLOW RATE $\left[\frac{\text { vol }}{\text { time }}\right]$
$V_{1}=\frac{A_{2} V_{2}}{A_{1}} \rightarrow$ COMBINE WITH BERNOULI $\rightarrow$ CRITICAL PATH
$\frac{2\left(p_{1}-p_{2}\right)}{\rho}=V_{2}^{2}-V_{1}^{2}=V_{2}^{2}-\left(\frac{A_{2} V_{2}}{A_{1}}\right)^{2}$

$\frac{2\left(p_{1}-p_{2}\right)}{\rho}=V_{2}^{2}\left(1-\left(\frac{A_{2}}{A_{1}}\right)^{2}\right)$
$\frac{\frac{2\left(p_{1}-p_{2}\right)}{\rho}}{\left(1-\left(\frac{A_{2}}{A_{1}}\right)^{2}\right)}=V_{2}^{2}$

$Q=A_{2} V_{2} \rightarrow$ VOLUME FLOW RATE DEFINITION

$$
Q\left[\frac{m^{3}}{s}\right]=A_{2}\left[m^{2}\right] \sqrt{\frac{2\left(p_{1}-p_{2}\right)\left[\frac{N=\frac{k \delta-m}{s^{2}}}{m^{2}}\right.}{\rho\left[\frac{k \delta}{m^{3}}\right]\left[1-\left(A_{2} / A_{1}\right)^{2}\right]}}\left[m^{2} \sqrt{\frac{m^{4}}{m^{2}-s^{2}}}=\frac{m^{3}}{s}\right]
$$




## ENERGY FLOW UNITS

| POWER $[$ Watts $]$ | $=\dot{m}\left[\frac{\mathrm{~kg}}{\mathrm{~s}}\right] \bullet g\left[\frac{\mathrm{~m}}{\mathrm{~s}^{2}}\right] \bullet h[\mathrm{~m}]$ |
| ---: | :--- |
|  | $=\frac{N-m}{s}$ |
|  | $=\frac{J}{s}$ |
|  | $=$ Watts |



## Conservation of Mass - Example

Problem \# 3.52: Water flows through the pipe contraction shown in Figure below. For the given 0.2 m difference in the manometer level, determine the flowrate as a function of the diameter of the small pipe, D.

Solution: Apply Bernoulli at $P_{1}$ and $P_{2}$.
$\dot{m}_{1}\left(P_{1}+\frac{1}{2} \rho V_{1}^{2}+\gamma Z_{1}\right)=\dot{m}_{2}\left(P_{2}+\frac{1}{2} \rho V_{2}^{2}+\gamma Z_{2}\right)$
SINGLE STREAM $\rightarrow \dot{m}_{1}=\dot{m}_{2}$
( $Z_{1}=Z_{2}$; same elevation)
$\underset{\text { Now }}{\Rightarrow} \frac{P_{1}-P_{2}}{\gamma}=\frac{V_{2}^{2}-V_{1}^{2}}{2 g}$

$$
Q_{1}=Q_{2} \rightarrow \text { MASS CONSERVATION }
$$

Mass Conservation is required when there is a change in flow AREA.

$$
V_{1} A_{1}=V_{2} A_{2} \rightarrow \text { CRITICAL FOR AREA CHANGE }
$$

$$
V_{2}=\frac{A_{1}}{A_{2}} V_{1}=\frac{\left(\pi / 4 D_{1}^{2}\right)}{\left(\pi / 4 D_{2}^{2}\right)} V_{1}=\left(\frac{0.1}{D}\right)^{2} V_{1}
$$



## Conservation of Mass - Example

Mass Conservation
$V_{2}=\left(\frac{0.1}{D}\right)^{2} V_{1}$;
Bernoulli
$\frac{P_{1}-P_{2}}{\gamma}=\frac{V_{2}^{2}-V_{1}^{2}}{2 g}$
Manometery
$P_{1}-\gamma h_{1}=0 \& P_{2}-\gamma h_{2}=0 \rightarrow($ POINT - TO - POINT $)$
$\Rightarrow \frac{P_{1}-P_{2}}{\gamma}=h_{1}-h_{2}=0.2 m$
Combine

$$
\begin{aligned}
& 0.2 m=\frac{(0.1 m / D)^{4} V_{1}^{2}-V_{1}^{2}}{2 g} \\
& \Rightarrow V_{1}(D)=\sqrt{\frac{0.2 m(2 g)}{\left[(0.1 m / D)^{4}-1\right]}}
\end{aligned}
$$

## FINAL

$$
Q_{1}=V_{1} A_{1}=\frac{\pi}{4}(0.1 m)^{2} \sqrt{\frac{0.2 m\left[2 \times\left(9.81 \mathrm{~m} / \mathrm{s}^{2}\right)\right]}{\left[(0.1 \mathrm{~m} / D)^{4}-1\right]}}
$$

$$
\Rightarrow Q(D)=\frac{0.0156 D^{2}}{\sqrt{\left[(0.1)^{4}-D^{4}\right]}} \frac{m^{3}}{s}
$$



## Confined Flows - Mass Conservation

Problem \# 3.68: Water flows steadily from the large open tank shown in the Figure below. If the viscous effects are negligible, determine (a) the flowrate, $Q$, and (b) the manometer reading, $\boldsymbol{h}$.
Solution: Apply Bernoulli along a streamline through $\mathbf{P}_{1}$ and $\mathbf{P}_{2}$.
Bernoulli
$\dot{m}_{1}\left(P_{1}+\frac{1}{2} \rho V_{1}^{2}+\gamma Z_{1}\right)=\dot{m}_{2}\left(P_{2}+\frac{1}{2} \rho V_{2}^{2}+\gamma Z_{2}\right)$

$\left(Z_{1}=4 m ; Z_{2} \approx 0 ; P_{1}=0 ; P_{2}=0 ; V_{1}=0, \dot{m}_{1}=\dot{m}_{2}\right)^{0.10 \mathrm{~m}}$
$\Rightarrow V_{2}=\sqrt{2 g Z_{1}}=\sqrt{2\left(9.8 \mathrm{~m} / \mathrm{s}^{2}\right)(4 \mathrm{~m})}=8.86 \mathrm{~m} / \mathrm{s}$
VOLUME FLOW RATE $\rightarrow$ DEFINITION

$$
Q=V_{2} A_{2}=(8.86 \mathrm{~m} / \mathrm{s}) \times \frac{\pi}{4}(0.1 \mathrm{~m})^{2}=0.0696 \mathrm{~m}^{3} / \mathrm{s}
$$

## Confined Flows - Mass Conservation

(b) Consider point $P_{3}$ at the same elevation of $P_{2}$. Apply Bernoulli along a streamline through $P_{2}$ and $P_{3}$.

$$
\begin{aligned}
& \dot{m}_{2}\left(P_{2}+\frac{1}{2} \rho V_{2}^{2}+\gamma Z_{2}\right)=\dot{m}_{3}\left(P_{3}+\frac{1}{2} \rho V_{3}^{2}+\gamma Z_{3}\right) \\
& \left(Z_{3}=Z_{2}, \text { same elevation; } P_{2}=0, \dot{m}_{2}=\dot{m}_{3} ;\right) \\
& \Rightarrow P_{3}=\frac{1}{2} \rho\left(V_{2}^{2}-V_{3}^{2}\right) \\
& \text { Now } \\
& Q_{2}=Q_{3} \\
& \Rightarrow V_{2} A_{2}=V_{3} A_{3} \\
& \Rightarrow V_{3}=\frac{A_{2}}{A_{3}} V_{2}=\left(\frac{D_{2}}{D_{3}}\right)^{2} V_{2}=\left(\frac{0.1 m}{0.08 m}\right)^{2}(8.86 \mathrm{~m} / \mathrm{s})=13.84 \mathrm{~m} / \mathrm{s}
\end{aligned}
$$

Therefore

$$
P_{3}=\frac{1}{2}\left(1000 \mathrm{~kg} / \mathrm{m}^{3}\right)\left[(8.86 \mathrm{~m} / \mathrm{s})^{2}-(13.84 \mathrm{~m} / \mathrm{s})^{2}\right]=-56,523 \mathrm{~N} / \mathrm{m}^{2}
$$

## Confined Flows - Mass Conservation

(b) Use manometry (APPLY BERRY METHOD CORRECTLY):
$P_{3}-\gamma_{H_{2} O}(2 m+(0.08 / 2) m)+\gamma_{H_{g}} h=P_{0}=0$
$h=\frac{\gamma_{H_{2} O}(2 m+(0.08 / 2) m)-P_{3}}{\gamma_{H_{g}}}$

0.10 m

OPEN
$=\frac{\left(9800 \mathrm{~N} / \mathrm{m}^{2}\right)(2.04 \mathrm{~m})-\left(-56,523 \mathrm{~N} / \mathrm{m}^{2}\right)}{13.55 \bullet 9800 \mathrm{~N} / \mathrm{m}^{2}}$
$\Rightarrow h=0.575 \mathrm{~m}$

$$
\left.N / m^{2}\right)
$$



WHAT IS PRESURE HERE ?
(5 Points)

## CONTINUITY \& BRANCH FLOWS

Water flows steadily through the horizontal pipe. If viscous effects are neglected determine water speed at section 2 , pressure at 3 , and flow rate at 4 .

Fluid Fundamentals

1. Invisid steady flow along multiple streamlines--BERNOULLI
2. Single input, multiple exit streams--CONTINUITY

$$
\begin{gathered}
A_{2}=0.07 \mathrm{ft}^{2} \\
p_{2}=5.0 \mathrm{psi}
\end{gathered}
$$

(2)

$$
\begin{aligned}
& A_{3}=0.2 \mathrm{ft}^{2} \\
& V_{3}=20 \mathrm{ft} / \mathrm{s}
\end{aligned}
$$

$$
\begin{equation*}
A_{1}=1 \mathrm{ft}^{2} \tag{3}
\end{equation*}
$$

$$
Q_{1}=10 \mathrm{ft}^{3} / \mathrm{s}
$$

PROB 3-112V8

$$
p_{1}=10 \mathrm{psi}
$$

## SOLUTION

FIND V2?
Apply Bernoulli to 1-2 STREAMLINE, $\mathrm{Z}_{1}=\mathrm{Z}_{2}$

$$
\begin{aligned}
& \dot{m}_{1}\left(\frac{P_{1}}{\gamma_{H 20}}+\frac{V_{1}^{2}}{2 g}\right)=\dot{m}_{2}\left(\frac{P_{2}}{\gamma_{H 20}}+\frac{V_{2}^{2}}{2 g}\right) \\
& \frac{\dot{m}_{1}}{\dot{m}_{2}}\left(\frac{P_{1}}{\gamma_{H 20}}+\frac{V_{1}^{2}}{2 g}\right)-\frac{P_{2}}{\gamma_{H 20}}=\frac{V_{2}^{2}}{2 g} \\
& \frac{A_{1} V_{1}}{A_{2} V_{2}}\left(\frac{P_{1}}{\gamma_{H 20}}+\frac{V_{1}^{2}}{2 g}\right)=\frac{P_{2}}{\gamma_{H 20}}+\frac{V_{2}^{2}}{2 g} \\
& \frac{A_{1} V_{1}}{A_{2}}\left(\frac{P_{1}}{\gamma_{H 20}}+\frac{V_{1}^{2}}{2 g}\right)=\frac{P_{2} V_{2}}{\gamma_{H 20}}+\frac{V_{2}^{3}}{2 g} \\
& V_{2}^{3}+V_{2}\left(\frac{P_{2}}{\gamma_{H 20}} \bullet 2 g\right)-\frac{A_{1} V_{1}}{A_{2}}\left(\frac{P_{1}}{\gamma_{H 20}}+\frac{V_{1}^{2}}{2 g}\right) \bullet 2 g=0
\end{aligned}
$$

$$
\begin{aligned}
& V_{1}=\frac{Q}{A_{1}} \\
& P_{2}=5 \frac{l b f}{i n^{2}} \cdot \frac{144 i n^{2}}{f t^{2}} \\
& P_{1}=10 \frac{l b f}{i n^{2}} \cdot \frac{144 i n^{2}}{f t^{2}} \\
& \gamma_{H_{2} \mathrm{O}}=62.4 \frac{l b f}{f t^{3}}
\end{aligned}
$$


$V_{2}^{3}+\left(0 V_{2}^{2}\right)+V_{2} q+r=0 \rightarrow$ ONE REAL ROOT
$a=\left(\frac{P_{2}}{\gamma_{H 20}} \bullet 2 g\right), b=-\frac{A_{1} V_{1}}{A_{2}}\left(\frac{P_{1}}{\gamma_{H 20}}+\frac{V_{1}^{2}}{2 g}\right) \bullet 2 g$
$A=\left(\frac{-b}{2}+\sqrt{\frac{b^{2}}{4}+\frac{a^{3}}{27}}\right)^{\frac{1}{3}} ; B=\left(\frac{-b}{2}-\sqrt{\frac{b^{2}}{4}+\frac{a^{3}}{27}}\right)^{\frac{1}{3}}$
root $=V_{2}=\mathrm{A}+\mathrm{B}$
$\mathrm{Q}_{2}\left[\frac{f t^{3}}{s}\right]=V_{2}\left[\frac{f t}{s}\right] \bullet A_{2}\left[f t^{3}\right]$

CARDANO'S FORMULA
(1501-1576)

## ANOTHER INSPIRING ENGINEERING MOVIE



## FIND P3

## FIND P3?

Apply Bernoulli to 1-3 STREAMLINE, $Z_{1}=Z_{3}$
$\dot{m}_{1}\left(\frac{P_{1}}{\gamma_{H 20}}+\frac{V_{1}^{2}}{2 g}\right)=\dot{m}_{3}\left(\frac{P_{3}}{\gamma_{H 20}}+\frac{V_{3}^{2}}{2 g}\right)$
$\left\{\frac{\dot{m}_{1}}{\dot{m}_{3}}\left(\frac{P_{1}}{\gamma_{H 20}}+\frac{V_{1}^{2}}{2 g}\right)-\frac{V_{3}^{2}}{2 g}\right\} \gamma_{H 20}=P_{3}$

$\left\{\frac{A_{1} V_{1}}{A_{3} V_{3}}\left(\frac{P_{1}}{\gamma_{H 20}}+\frac{V_{1}^{2}}{2 g}\right)\left[\frac{\frac{l b f}{f t^{2}}}{\frac{l b f}{f t^{3}}}=f t\right]-\frac{V_{3}^{2}}{2 g}\right\} \gamma_{H 20}\left[\frac{l b f}{f t^{3}}\right]=P_{3}\left[\frac{l b f}{f t^{2}}\right]$
Mass Conservation $\rightarrow \mathrm{V}_{1}=\frac{Q_{1}}{A_{1}}=10 \mathrm{ft} / \mathrm{s}$

## FIND Q4

## STEADY STATE

$\sum \dot{m}_{o m}-\sum \dot{m}_{m}=0$
$\sum(\rho A V)_{o u t}-\sum(\rho A V)_{i n}=0$
$\sum(\rho Q)_{o u t}-\sum(\rho Q)_{\text {in }}=0$
$\sum(Q)_{\text {out }}-\sum(Q)_{i n}=0$
$Q_{2}+Q_{3}+Q_{4}-Q_{1}=0$
Solution
$Q_{4}=Q_{1}-Q_{2}-Q_{3}$

$Q_{4}=10 \frac{f t^{3}}{s}-0.70 \frac{f t^{3}}{s}-4 \frac{f t^{3}}{s}$
$Q_{4}=5.3 \frac{\mathrm{ft}^{3}}{\mathrm{~s}}$


Figure P3. 104
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FIND FLOW RATE and PRESSURE at Point 1 Inviscid and Steady Flow 3-113V8

Water flows from the pipe as "free jet" and strikes a circular flat plate. Determine the flowrate and the manometer reading H


## BERNOULLI

## Bernoulli 1-3

$\dot{m}_{1}\left(\frac{P_{1}}{\gamma_{H 2 D}}+Z_{1}+\frac{V_{1}^{2}}{2 g}\right)=\dot{m}_{3}\left(\frac{P_{3}}{\gamma_{H 2 D}}+Z_{3}+\frac{V_{3}^{2}}{2 g}\right)$
$V_{1}=1.6 V_{3}($ Mass Conservation $), \dot{m}_{1}=\dot{m}_{3}$
$\frac{\left(1.6 V_{3}\right)^{2}}{2 g}-\frac{V_{3}^{2}}{2 g}=Z_{3}-Z_{1}$
$\frac{V_{3}^{2}}{2 g}\left(1.6^{2}-1\right)=0.2, g=9.81 \mathrm{~m} / \mathrm{s}^{2}$
$1.59 \mathrm{~m} / \mathrm{s}=V_{3} \rightarrow V_{1}=1.6 V_{3}=2.5 \mathrm{~m} / \mathrm{s}$
Volume Flow Rate
$\mathrm{Q}=\mathrm{VA}=\mathrm{V}_{1} \mathrm{~A}_{1}=2.5 \mathrm{~m} / \mathrm{s} \frac{\pi 0.01^{2}}{4}=0.0002 \mathrm{~m}^{3} / \mathrm{s}$


## MANOMETER

Bernoulli 1-2
$\dot{\eta}_{1}\left(\frac{P_{1}}{\gamma_{H 22}}+Z_{1}+\frac{V_{1}^{2}}{2 g}\right)=\dot{\dot{g}_{2}}\left(\frac{P_{2}}{\gamma_{H 20}}+Z_{2}+\frac{\underline{X}_{2}^{2}}{2 g}\right)$
$P_{2}=P_{S T A G}=\left(\frac{V_{1}^{2}}{2 g}+Z_{1}-Z_{2}\right) \gamma_{H 20}, \dot{m}_{1}=\dot{m}_{2}$
$V_{1}=1.6 V_{3}=2.5 \mathrm{~m} / \mathrm{s}$
$P_{2}=P_{S T A G}=\left(\frac{2.54^{2} m / s}{2 \bullet 9.81 m / s^{2}}-0.20 m\right) \gamma_{H 20} \frac{N}{m^{3}}$
$=1263 \mathrm{PA}$
Manometer 2-0

$$
P_{2}-\gamma_{H 20} H=P_{0}=0 \text { gauge }
$$



$$
H=\frac{P_{2}}{\gamma_{H 20}}=\frac{1263 \mathrm{~N} / \mathrm{m}^{2}}{9800 \mathrm{~N} / \mathrm{m}^{3}}=0.13 \mathrm{~m}
$$

## HOMEWORK

## 3-96,97,99,109,111,113,108,109

## End of Chapter 3

任何人都可以记住事情，但重要的是要了解它。

